

# MANE 4240 & CIVL 4240

## Introduction to Finite Elements

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# Four-noded rectangular element

## **Reading assignment:**

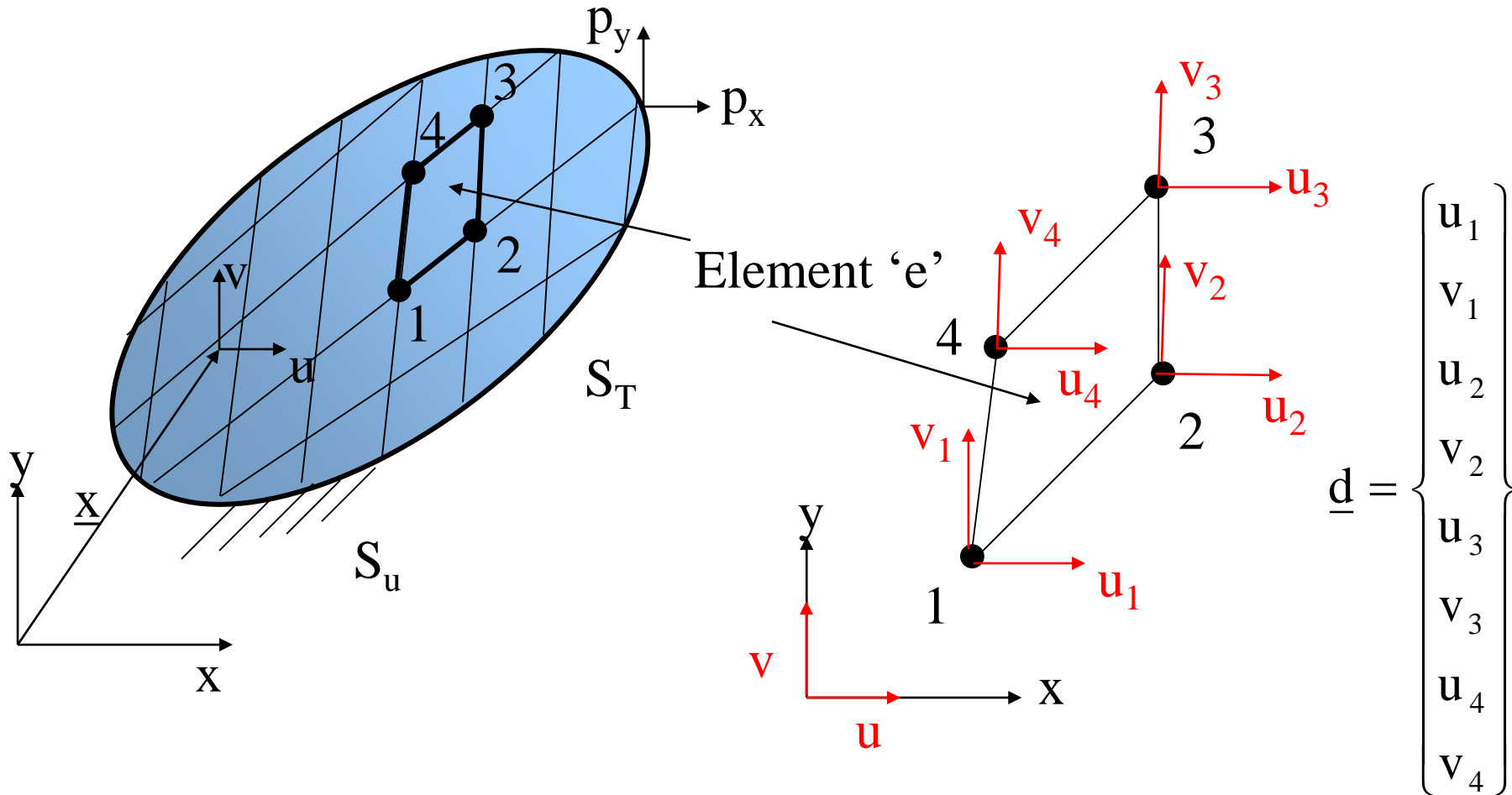
**Logan 10.2 + Lecture notes**

## **Summary:**

- Computation of shape functions for 4-noded quad
- Special case: rectangular element
- Properties of shape functions
- Computation of strain-displacement matrix
- Example problem
- Hint at how to generate shape functions of higher order (Lagrange) elements

## Finite element formulation for 2D:

**Step 1:** Divide the body into **finite elements** connected to each other through special points (“**nodes**”)



## Summary: For each element

**Displacement approximation** in terms of shape functions

$$\underline{u} = \underline{N} \underline{d}$$

**Strain approximation** in terms of strain-displacement matrix

$$\underline{\varepsilon} = \underline{B} \underline{d}$$

**Stress approximation**

$$\underline{\sigma} = \underline{D} \underline{B} \underline{d}$$

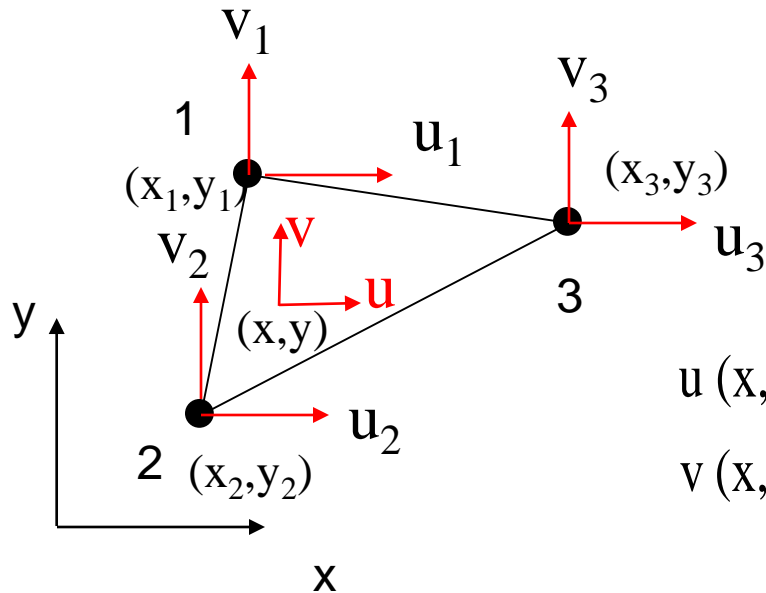
**Element stiffness matrix**

$$\underline{k} = \int_{V^e} \underline{B}^T \underline{D} \underline{B} dV$$

**Element nodal load vector**

$$\underline{f} = \underbrace{\int_{V^e} \underline{N}^T \underline{X} dV}_{\underline{f}_b} + \underbrace{\int_{S_T^e} \underline{N}^T \underline{T}_S dS}_{\underline{f}_s}$$

## Constant Strain Triangle (CST) : Simplest 2D finite element

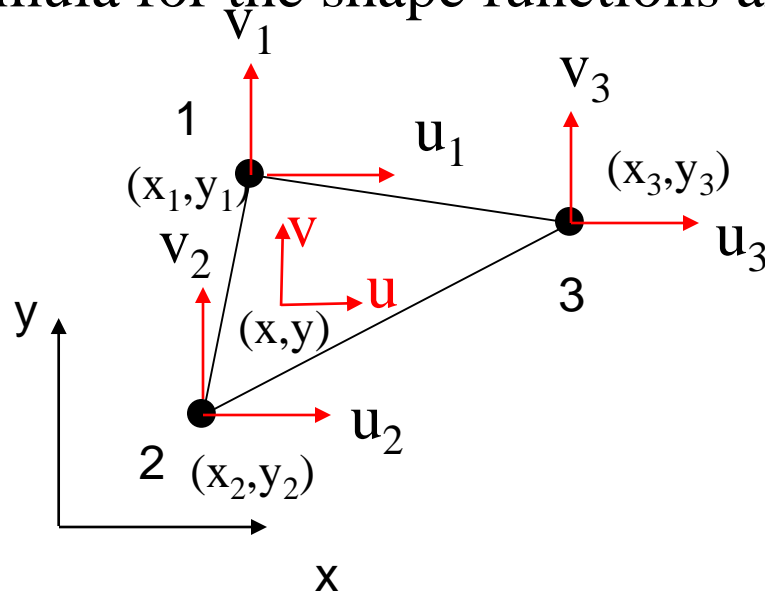


$$u(x, y) \approx N_1(x, y) u_1 + N_2(x, y) u_2 + N_3(x, y) u_3$$

$$v(x, y) \approx N_1(x, y) v_1 + N_2(x, y) v_2 + N_3(x, y) v_3$$

- 3 nodes per element
- 2 dofs per node (each node can move in x- and y- directions)
- Hence 6 dofs per element

Formula for the shape functions are



where

$$N_1 = \frac{a_1 + b_1x + c_1y}{2A}$$

$$N_2 = \frac{a_2 + b_2x + c_2y}{2A}$$

$$N_3 = \frac{a_3 + b_3x + c_3y}{2A}$$

$$A = \text{area of triangle} = \frac{1}{2} \det \begin{bmatrix} 1 & x_1 & y_1 \\ 1 & x_2 & y_2 \\ 1 & x_3 & y_3 \end{bmatrix}$$

$$a_1 = x_2 y_3 - x_3 y_2 \quad b_1 = y_2 - y_3 \quad c_1 = x_3 - x_2$$

$$a_2 = x_3 y_1 - x_1 y_3 \quad b_2 = y_3 - y_1 \quad c_2 = x_1 - x_3$$

$$a_3 = x_1 y_2 - x_2 y_1 \quad b_3 = y_1 - y_2 \quad c_3 = x_2 - x_1$$

## Approximation of displacements

$$\underline{\mathbf{u}} = \underline{\mathbf{N}} \underline{\mathbf{d}}$$

$$\underline{\mathbf{u}} = \begin{Bmatrix} \mathbf{u}(x, y) \\ \mathbf{v}(x, y) \end{Bmatrix} = \begin{bmatrix} \mathbf{N}_1 & 0 & \mathbf{N}_2 & 0 & \mathbf{N}_3 & 0 \\ 0 & \mathbf{N}_1 & 0 & \mathbf{N}_2 & 0 & \mathbf{N}_3 \end{bmatrix} \begin{Bmatrix} \mathbf{u}_1 \\ \mathbf{v}_1 \\ \mathbf{u}_2 \\ \mathbf{v}_2 \\ \mathbf{u}_3 \\ \mathbf{v}_3 \end{Bmatrix}$$

## Approximation of the strains

$$\underline{\boldsymbol{\varepsilon}} = \begin{Bmatrix} \boldsymbol{\varepsilon}_x \\ \boldsymbol{\varepsilon}_y \\ \boldsymbol{\gamma}_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial \mathbf{u}}{\partial x} \\ \frac{\partial \mathbf{v}}{\partial y} \\ \frac{\partial \mathbf{u}}{\partial y} + \frac{\partial \mathbf{v}}{\partial x} \end{Bmatrix} \approx \underline{\mathbf{B}} \underline{\mathbf{d}}$$

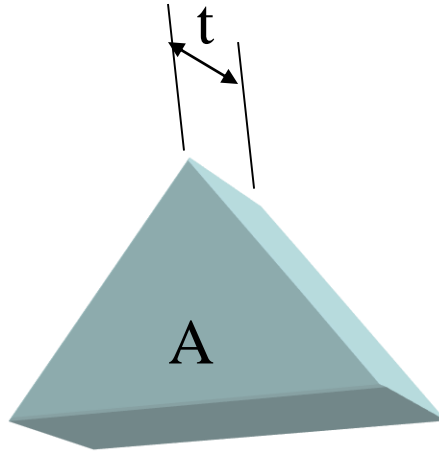
$$\underline{\mathbf{B}} = \begin{bmatrix} \frac{\partial \mathbf{N}_1(x, y)}{\partial x} & 0 & \frac{\partial \mathbf{N}_2(x, y)}{\partial x} & 0 & \frac{\partial \mathbf{N}_3(x, y)}{\partial x} & 0 \\ 0 & \frac{\partial \mathbf{N}_1(x, y)}{\partial y} & 0 & \frac{\partial \mathbf{N}_2(x, y)}{\partial y} & 0 & \frac{\partial \mathbf{N}_3(x, y)}{\partial y} \\ \frac{\partial \mathbf{N}_1(x, y)}{\partial y} & \frac{\partial \mathbf{N}_1(x, y)}{\partial x} & \frac{\partial \mathbf{N}_2(x, y)}{\partial y} & \frac{\partial \mathbf{N}_2(x, y)}{\partial x} & \frac{\partial \mathbf{N}_3(x, y)}{\partial y} & \frac{\partial \mathbf{N}_3(x, y)}{\partial x} \end{bmatrix} = \frac{1}{2A} \begin{bmatrix} b_1 & 0 & b_2 & 0 & b_3 & 0 \\ 0 & c_1 & 0 & c_2 & 0 & c_3 \\ c_1 & b_1 & c_2 & b_2 & c_3 & b_3 \end{bmatrix}$$

## Element stiffness matrix

$$\underline{k} = \int_{V^e} \underline{B}^T \underline{D} \underline{B} dV$$

Since  $\underline{B}$  is constant

$$\underline{k} = \underline{B}^T \underline{D} \underline{B} \int_{V^e} dV = \underline{B}^T \underline{D} \underline{B} A t$$



t=thickness of the element

A=surface area of the element

## Element nodal load vector

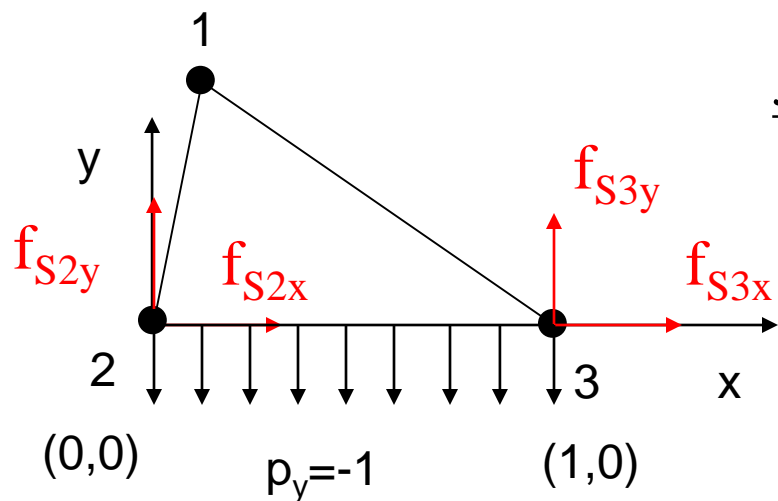
$$\underline{f} = \underbrace{\int_{V^e} \underline{N}^T \underline{X} dV}_{\underline{f}_b} + \underbrace{\int_{S_T^e} \underline{N}^T \underline{T}_s dS}_{\underline{f}_s}$$



## Class exercise

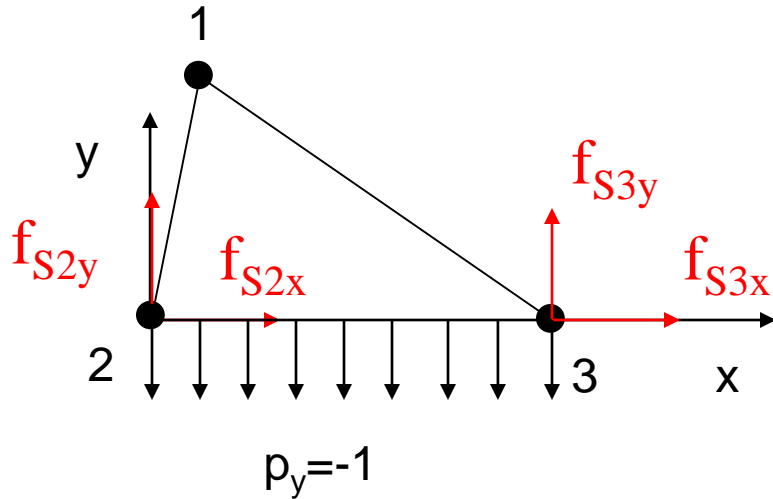
For the CST shown below, compute the vector of nodal loads due to surface traction

$$\underline{f}_S = \int_{S_T^e} \underline{N}^T \underline{T}_S dS$$



$$\underline{f}_S = t \int_{l_{1-3}^e} \underline{N}^T \Big|_{\text{along } 2-3} \underline{T}_S dS$$

# Class exercise



$$\underline{f}_{-S} = t \int_{l_{1-3}} \underline{N}^T \Big|_{\text{along } 2-3} \underline{T}_{-S} dS$$

$$\underline{T}_{-S} = \begin{Bmatrix} 0 \\ -1 \end{Bmatrix}$$

The only nonzero nodal loads are

$$f_{S_{2y}} = t \int_{x=0}^1 N_2 \Big|_{\text{along } 2-3} p_y dx$$

$$f_{S_{3y}} = t \int_{x=0}^1 N_3 \Big|_{\text{along } 2-3} p_y dx$$

$$N_2 \Big|_{\text{along } 2-3} = \left[ \frac{a_2 + b_2 x + c_2 y}{2A} \right]_{y=0} = \frac{a_2 + b_2 x}{2A} = \frac{(x_3 y_1 - x_1 y_3) + (y_3 - y_1)x}{2A}$$

$$= \frac{y_1 - y_1 x}{\det \begin{bmatrix} 1 & x_1 & y_1 \\ 1 & x_2 & y_2 \\ 1 & x_3 & y_3 \end{bmatrix}} = \frac{y_1(1-x)}{\det \begin{bmatrix} 1 & x_1 & y_1 \\ 1 & x_2 & 0 \\ 1 & x_3 & 0 \end{bmatrix}} = \frac{y_1(1-x)}{y_1(x_3 - x_2)}$$

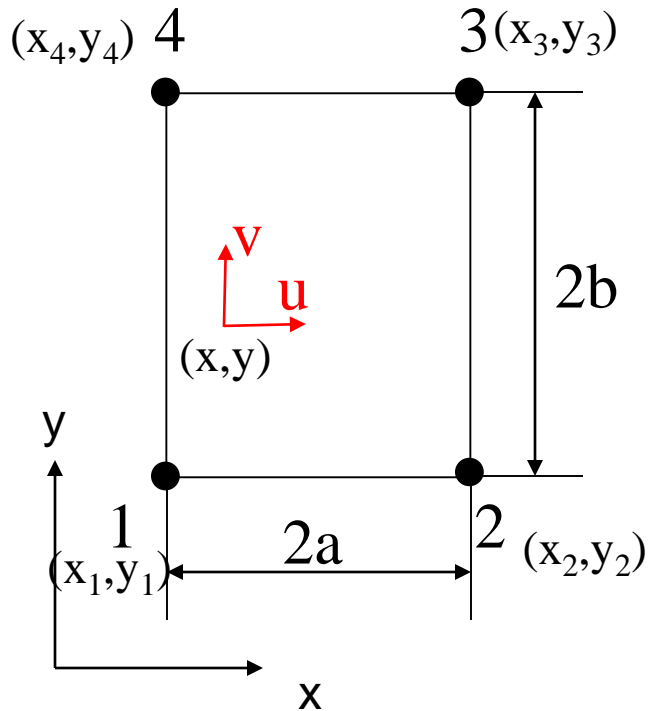
$$= 1 - x \quad (\text{can you derive this simpler?})$$

$$\begin{aligned}\Rightarrow f_{S_{2y}} &= t \int_{x=0}^1 N_2|_{\text{along } 2-3} p_y dx \\ &= t \int_{x=0}^1 (1-x)(-1) dx \\ &= -\frac{t}{2}\end{aligned}$$

Now compute

$$f_{S_{3y}} = t \int_{x=0}^1 N_3|_{\text{along } 2-3} p_y dx$$

## 4-noded rectangular element with edges parallel to the coordinate axes:



$$u(x, y) \approx \sum_{i=1}^4 N_i(x, y) u_i$$
$$v(x, y) \approx \sum_{i=1}^4 N_i(x, y) v_i$$

- 4 nodes per element
- 2 dofs per node (each node can move in x- and y- directions)
- 8 dofs per element

Using similar arguments, choose

$$N_1 = \frac{1}{4ab}(x - x_2)(y - y_4)$$

$$N_2 = -\frac{1}{4ab}(x - x_1)(y - y_3)$$

$$N_3 = \frac{1}{4ab}(x - x_4)(y - y_2)$$

$$N_4 = -\frac{1}{4ab}(x - x_3)(y - y_1)$$

## Properties of the shape functions:

**1. The shape functions  $N_1, N_2, N_3$  and  $N_4$  are bilinear functions of  $x$  and  $y$**

**2. Kronecker delta property**

$$N_i(x, y) = \begin{cases} 1 & \text{at node 'i'} \\ 0 & \text{at other nodes} \end{cases}$$

**3. Completeness**

$$\sum_{i=1}^4 N_i = 1$$
$$\sum_{i=1}^4 N_i x_i = x$$
$$\sum_{i=1}^4 N_i y_i = y$$

- 3. Along lines parallel to the x- or y-axes, the shape functions are linear. But along any other line they are nonlinear.**
- 4. An element shape function related to a specific nodal point is zero along element boundaries not containing the nodal point.**
- 5. The displacement field is continuous across elements**
- 6. The strains and stresses are not constant within an element nor are they continuous across element boundaries.**

# The strain-displacement relationship

$$\underline{\varepsilon} = \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

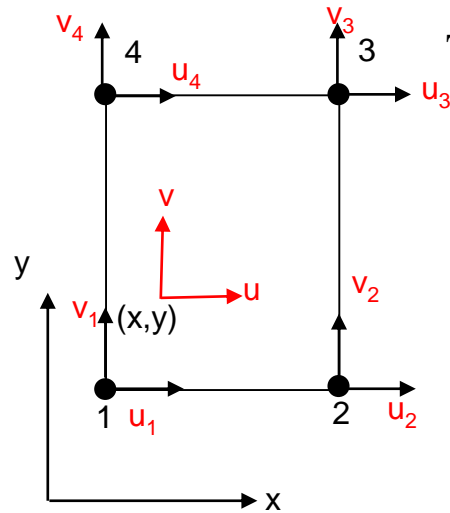
$$= \begin{bmatrix} \frac{\partial N_1(x,y)}{\partial x} & 0 & \frac{\partial N_2(x,y)}{\partial x} & 0 & \frac{\partial N_3(x,y)}{\partial x} & 0 & \frac{\partial N_4(x,y)}{\partial x} & 0 \\ 0 & \frac{\partial N_1(x,y)}{\partial y} & 0 & \frac{\partial N_2(x,y)}{\partial y} & 0 & \frac{\partial N_3(x,y)}{\partial y} & 0 & \frac{\partial N_4(x,y)}{\partial y} \\ \frac{\partial N_1(x,y)}{\partial y} & \frac{\partial N_1(x,y)}{\partial x} & \frac{\partial N_2(x,y)}{\partial y} & \frac{\partial N_2(x,y)}{\partial x} & \frac{\partial N_3(x,y)}{\partial y} & \frac{\partial N_3(x,y)}{\partial x} & \frac{\partial N_4(x,y)}{\partial y} & \frac{\partial N_4(x,y)}{\partial x} \end{bmatrix} \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ u_3 \\ v_3 \\ u_4 \\ v_4 \end{Bmatrix}$$

$$\underline{B} = \frac{1}{4ab} \begin{bmatrix} y - y_4 & 0 & y_3 - y & 0 & y - y_2 & 0 & y_1 - y & 0 \\ 0 & x - x_2 & 0 & x_1 - x & 0 & x - x_4 & 0 & x_3 - x \\ x - x_2 & y - y_4 & x_1 - x & y_3 - y & x - x_4 & y - y_2 & x_3 - x & y_1 - y \end{bmatrix}$$

**Notice** that the strains (and hence the stresses) are NOT constant within an element



## Computation of the terms in the stiffness matrix of 2D elements (recap)



The  $\underline{\mathbf{B}}$ -matrix (strain-displacement) corresponding to this element is

$u_1$	$v_1$	$u_2$	$v_2$	$u_3$	$v_3$	$u_4$	$v_4$
$\frac{\partial N_1(x,y)}{\partial x}$	0	$\frac{\partial N_2(x,y)}{\partial x}$	0	$\frac{\partial N_3(x,y)}{\partial x}$	0	$\frac{\partial N_4(x,y)}{\partial x}$	0
0	$\frac{\partial N_1(x,y)}{\partial y}$	0	$\frac{\partial N_2(x,y)}{\partial y}$	0	$\frac{\partial N_3(x,y)}{\partial y}$	0	$\frac{\partial N_4(x,y)}{\partial y}$
$\frac{\partial N_1(x,y)}{\partial y}$	$\frac{\partial N_1(x,y)}{\partial x}$	$\frac{\partial N_2(x,y)}{\partial y}$	$\frac{\partial N_2(x,y)}{\partial x}$	$\frac{\partial N_3(x,y)}{\partial y}$	$\frac{\partial N_3(x,y)}{\partial x}$	$\frac{\partial N_4(x,y)}{\partial y}$	$\frac{\partial N_4(x,y)}{\partial x}$

We will denote the columns of the  $\underline{\mathbf{B}}$ -matrix as

$$\underline{\mathbf{B}}_{u_1} = \begin{bmatrix} \frac{\partial N_1(x,y)}{\partial x} \\ 0 \\ \frac{\partial N_1(x,y)}{\partial y} \end{bmatrix}; \underline{\mathbf{B}}_{v_1} = \begin{bmatrix} 0 \\ \frac{\partial N_1(x,y)}{\partial y} \\ \frac{\partial N_1(x,y)}{\partial x} \end{bmatrix}; \text{ and so on...}$$

The **stiffness matrix** corresponding to this element is

$$\underline{k} = \int_{V^e} \underline{B}^T \underline{D} \underline{B} dV$$

which has the following form

	$u_1$	$v_1$	$u_2$	$v_2$	$u_3$	$v_3$	$u_4$	$v_4$	
$\underline{k} =$	$k_{11}$	$k_{12}$	$k_{13}$	$k_{14}$	$k_{15}$	$k_{16}$	$k_{17}$	$k_{18}$	$u_1$
	$k_{21}$	$k_{22}$	$k_{23}$	$k_{24}$	$k_{25}$	$k_{26}$	$k_{27}$	$k_{28}$	$v_1$
	$k_{31}$	$k_{32}$	$k_{33}$	$k_{34}$	$k_{35}$	$k_{36}$	$k_{37}$	$k_{38}$	$u_2$
	$k_{41}$	$k_{42}$	$k_{43}$	$k_{44}$	$k_{45}$	$k_{46}$	$k_{47}$	$k_{48}$	$v_2$
	$k_{51}$	$k_{52}$	$k_{53}$	$k_{54}$	$k_{55}$	$k_{56}$	$k_{57}$	$k_{58}$	$u_3$
	$k_{61}$	$k_{62}$	$k_{63}$	$k_{64}$	$k_{65}$	$k_{66}$	$k_{67}$	$k_{68}$	$v_3$
	$k_{71}$	$k_{72}$	$k_{73}$	$k_{74}$	$k_{75}$	$k_{76}$	$k_{77}$	$k_{78}$	$u_4$
	$k_{81}$	$k_{82}$	$k_{83}$	$k_{84}$	$k_{85}$	$k_{86}$	$k_{87}$	$k_{88}$	$v_4$

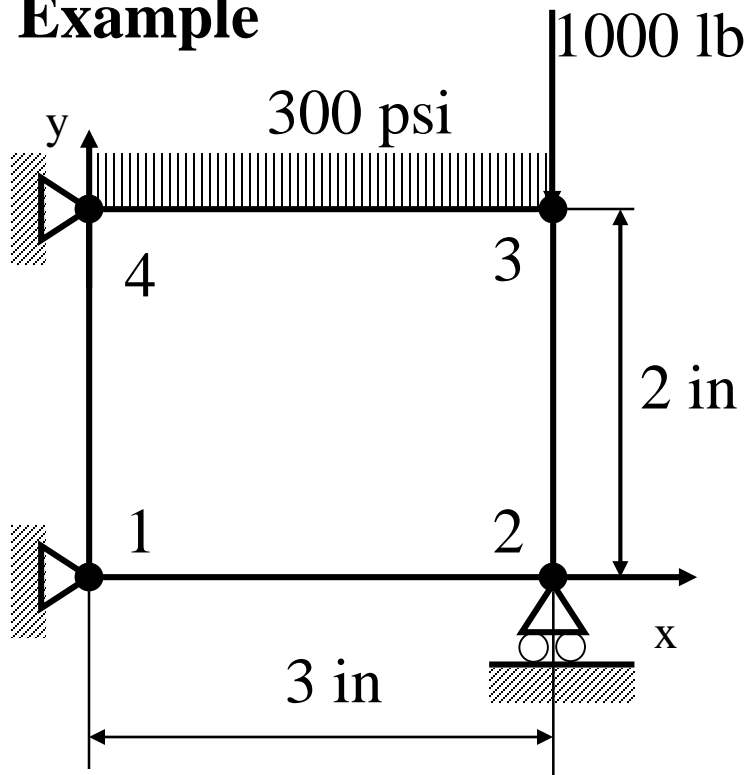
The individual entries of the stiffness matrix may be computed as follows

$$k_{11} = \int_{V^e} \underline{B}_{u_1}^T \underline{D} \underline{B}_{u_1} dV; \quad k_{12} = \int_{V^e} \underline{B}_{u_1}^T \underline{D} \underline{B}_{v_1} dV; \quad k_{13} = \int_{V^e} \underline{B}_{u_1}^T \underline{D} \underline{B}_{u_2} dV, \dots$$

$$k_{21} = \int_{V^e} \underline{B}_{v_1}^T \underline{D} \underline{B}_{u_1} dV; \quad k_{21} = \int_{V^e} \underline{B}_{v_1}^T \underline{D} \underline{B}_{v_1} dV; \dots$$

Notice that these formulae are quite general (apply to all kinds of finite elements, CST, quadrilateral, etc) since we have not used any specific shape functions for their derivation.

## Example



Thickness ( $t$ ) = 0.5 in  
 $E = 30 \times 10^6$  psi  
 $\nu = 0.25$

- Compute the unknown nodal displacements.
- Compute the stresses in the two elements.

This is exactly the same problem that we solved in last class, except now we have to use a **single** 4-noded element

**Realize that this is a plane stress problem and therefore we need to use**

$$\underline{D} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} = \begin{bmatrix} 3.2 & 0.8 & 0 \\ 0.8 & 3.2 & 0 \\ 0 & 0 & 1.2 \end{bmatrix} \times 10^7 \text{ psi}$$

**Write down the shape functions**

$$N_1 = \frac{1}{4ab} (x - x_2)(y - y_4) = \frac{(x-3)(y-2)}{6}$$

$$N_2 = -\frac{1}{4ab} (x - x_1)(y - y_3) = -\frac{x(y-2)}{6}$$

$$N_3 = \frac{1}{4ab} (x - x_4)(y - y_2) = \frac{xy}{6}$$

$$N_4 = -\frac{1}{4ab} (x - x_3)(y - y_1) = -\frac{(x-3)y}{6}$$

x	y
0	0
3	0
3	2
0	2

We have 4 nodes with 2 dofs per node=8dofs. However, 5 of these are fixed.  
The nonzero displacements are

$$u_2 \quad u_3 \quad v_3$$

Hence we need to solve

$$\begin{array}{c|c|c|c|c}
 u_2 & u_3 & v_3 & & \\
 \hline
 k_{11} & k_{12} & k_{13} & \left\{ \begin{array}{c} u_2 \\ u_3 \end{array} \right\} & \left\{ \begin{array}{c} 0 \\ 0 \end{array} \right\} \\
 \hline
 k_{21} & k_{22} & k_{23} & & \\
 \hline
 k_{31} & k_{32} & k_{33} & \left\{ \begin{array}{c} v_3 \end{array} \right\} & \left\{ \begin{array}{c} f_{3y} \end{array} \right\} \\
 \hline
 \end{array}$$

Need to compute only the **relevant terms in the stiffness matrix**

$$k_{11} = \int_{V^e} \underline{\mathbf{B}}_{u_2}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_2} dV; \quad k_{12} = \int_{V^e} \underline{\mathbf{B}}_{u_2}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_3} dV; \quad k_{13} = \int_{V^e} \underline{\mathbf{B}}_{u_2}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{v_3} dV$$

$$k_{21} = \int_{V^e} \underline{\mathbf{B}}_{u_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_2} dV; \quad k_{22} = \int_{V^e} \underline{\mathbf{B}}_{u_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_3} dV; \quad k_{23} = \int_{V^e} \underline{\mathbf{B}}_{u_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{v_3} dV$$

$$k_{31} = \int_{V^e} \underline{\mathbf{B}}_{v_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_2} dV; \quad k_{32} = \int_{V^e} \underline{\mathbf{B}}_{v_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_3} dV; \quad k_{33} = \int_{V^e} \underline{\mathbf{B}}_{v_3}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{v_3} dV$$

## Compute only the relevant columns of the B matrix

$$\underline{B}_{u_2} = \begin{Bmatrix} \frac{\partial N_2}{\partial x} \\ \mathbf{0} \\ \frac{\partial N_2}{\partial y} \end{Bmatrix} = \begin{Bmatrix} (2-y) \\ 6 \\ -\frac{x}{6} \end{Bmatrix}$$

$$\underline{B}_{u_3} = \begin{Bmatrix} \frac{\partial N_3}{\partial x} \\ \mathbf{0} \\ \frac{\partial N_3}{\partial y} \end{Bmatrix} = \begin{Bmatrix} y \\ 6 \\ x \\ 6 \end{Bmatrix}$$

$$\underline{B}_{v_3} = \begin{Bmatrix} \mathbf{0} \\ \frac{\partial N_3}{\partial y} \\ \frac{\partial N_3}{\partial x} \end{Bmatrix} = \begin{Bmatrix} \mathbf{0} \\ x \\ 6 \\ y \\ 6 \end{Bmatrix}$$

$$\begin{aligned}
k_{11} &= \int_{V^e} \underline{\mathbf{B}}_{u_2}^T \underline{\mathbf{D}} \underline{\mathbf{B}}_{u_2} dV \\
&= 0.5 \int_{x=0}^3 \int_{y=0}^2 \left[ (0.1067 \times 10^8 - 0.533 \times 10^7) \left( \frac{2-y}{6} \right) + 3.33 \times 10^5 x^2 \right] dx dy \\
&= 0.656 \times 10^7
\end{aligned}$$

Similarly compute the other terms



## How do we compute $f_{3y}$

$$f_{3y} = -1000 + f_{S_{3y}}$$

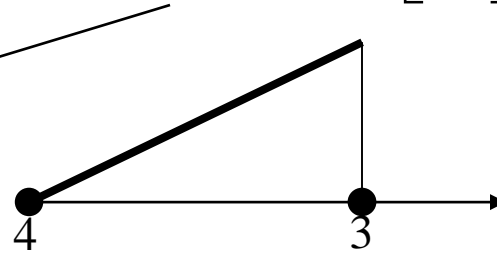
$$f_{S_{3y}} = t \int_{x=0}^3 N_3 \Big|_{\substack{\text{along} \\ \text{edge } 3-4}} (-300) dx$$

$$= (0.5)(-300) \int_{x=0}^3 \frac{x}{3} dx$$

$$= -150 \times \frac{3}{2}$$

$$= -225 \text{ lb}$$

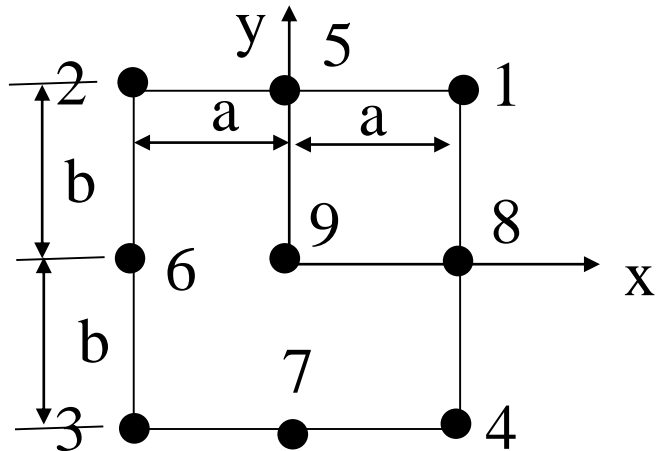
$$N_3 \Big|_{\substack{\text{edge} \\ 4-3}} = N_3 \Big|_{y=2} = \left[ \frac{xy}{6} \right]_{y=2} = \frac{x}{3}$$



$$\Rightarrow f_{3y} = -1000 + f_{S_{3y}} = -1225 \text{ lb}$$

How about a **9-noded rectangle**?

Corner nodes



$$N_1 = \left[ \frac{x(a+x)}{2a^2} \right] \left[ \frac{y(b+y)}{2b^2} \right] \quad N_2 = \left[ -\frac{x(a-x)}{2a^2} \right] \left[ \frac{y(b+y)}{2b^2} \right]$$

$$N_3 = \left[ -\frac{x(a-x)}{2a^2} \right] \left[ -\frac{y(b-y)}{2b^2} \right] \quad N_4 = \left[ \frac{x(a+x)}{2a^2} \right] \left[ -\frac{y(b-y)}{2b^2} \right]$$

Midside nodes

$$N_5 = \left[ \frac{a^2 - x^2}{a^2} \right] \left[ \frac{y(b+y)}{2b^2} \right] \quad N_6 = \left[ -\frac{x(a-x)}{2a^2} \right] \left[ \frac{b^2 - y^2}{b^2} \right]$$

$$N_7 = \left[ \frac{a^2 - x^2}{a^2} \right] \left[ -\frac{y(b-y)}{2b^2} \right] \quad N_8 = \left[ \frac{x(a+x)}{2a^2} \right] \left[ \frac{b^2 - y^2}{b^2} \right]$$

Center node

$$N_9 = \left[ \frac{a^2 - x^2}{a^2} \right] \left[ \frac{b^2 - y^2}{b^2} \right]$$

**Question:** Can you generate the shape functions of a 16-noded rectangle?

**Note:** These elements, whose shape functions are generated by multiplying the shape functions of 1D elements, are said to belong to the **“Lagrange” family**